

8 April 2015

Our ref: GENZWELL16067AB-AA

Progeni Limited
68 Exploration Way
Whitby
Wellington

Attention: David Harpham

Dear David,

**68 Exploration Way, Whitby
Earthworks Specification and Water Reservoir Subgrade Assessment**

1. INTRODUCTION

This report provides the Earthworks Specification (Appendix A) and Water Reservoir Platform Design Advice (Appendix B) as presented in our proposal GENZWELL16067AA-AC dated 4 July 2014 (Items 2 and 4), as part of the subdivision development at 68 Exploration Way, Whitby.

2. SITE INVESTIGATION

A geotechnical investigation has been undertaken at the site comprising excavation of five test pits to assess the properties of material underling the site. Test pits TP01, TP03, TP04 and TP05 have been excavated to a maximum depth of 2.5m below ground level (mbgl), at the locations of proposed cut slopes in order to allow slope stability assessments of the proposed cuts, and for sampling of the material for earthworks compaction testing. This report does not present an assessment of the stability of the proposed cuts.

Test Pit TP02 was excavated in the location of the proposed water reservoirs at the southern part of the site, to allow an assessment of the slope stability to ensure no adverse impacts on the water reservoir foundations occur.

The locations of the test pits and photographs of the test pits after excavation and of the spoil are contained within Appendix A.

The test pits revealed the cut slopes are located within Residual Greywacke soils overlain by between 0.2 to 0.5m of topsoil. The greywacke was dry and hard within test pits TP01 and TP03 and moist and

stiff to very stiff within TP04 and TP05. The test pit lithology, assessed consistency and undrained shear strength of the Residual Greywacke material based on shear vane testing within the test pits is presented below.

Table 1 Test Pit Lithology and Assessed Soil Strength

Test Pit	Lithology (mbgl)	Assessed Consistency	Peak Shear Strength / Residual Shear Strength (kPa)*
TP01	0.0-0.3: Topsoil.	Hard	>200
	0.3-2.5: Residual Greywacke, light grey/orange brown, fine sandy SILT, dry.		
TP02	0.0-0.5: Topsoil.	Hard	>200
	0.5-2.2: Residual Greywacke, orange brown, fine sandy SILT, dry.		
TP03	0.0-0.4: Topsoil.	Hard	>200
	0.4-1.75: Residual Greywacke, yellow brown, fine sandy SILT, dry.		
TP04	0.0-0.3: Topsoil.	Stiff	48-78 / 15
	0.3-1.7: Residual Greywacke, orange brown, fine sandy SILT, moist.		
TP05	0.0-0.2: Topsoil.	Very Stiff	117 – 128 / 28
	0.2-2.3: Residual Greywacke, orange grey, fine sandy SILT, moist.		

* Hand shear vane testing completed within Residual Greywacke only. No tests completed in topsoil.

3. FUTURE GEOTECHNICAL WORK

Future geotechnical advice for the subdivision development (refer Coffey Proposal 16067AA-AC dated 4 July 2014) includes:

1. Site inspections for certification of fill placement and Lot subgrade assessment (Section 2.2.1).
2. Stormwater Detention Basin Design (Section 2.2.3).
3. Final Geotechnical Report with Certification (Section 2.2.8).

Note that the stability of the proposed cuts to be steepened beyond the PCC guidelines adjacent Lots 7 and 8 can be undertaken for an additional fee of \$1,500 (+ GST) assuming that slope cross sections are provided by Progeni Limited.

For and on behalf of Coffey

Prepared By:



Andrew Hutchinson
Project Engineering Geologist

Reviewed/ Authorised By:



Kah-Weng Ho
Principal Geotechnical Engineer

Appendix A – Earthworks Specification

Appendix B – Water Reservoir Platform Assessment



Important information about your Coffey Report

As a client of Coffey you should know that site subsurface conditions cause more construction problems than any other factor. These notes have been prepared by Coffey to help you interpret and understand the limitations of your report.

Your report is based on project specific criteria

Your report has been developed on the basis of your unique project specific requirements as understood by Coffey and applies only to the site investigated. Project criteria typically include the general nature of the project; its size and configuration; the location of any structures on the site; other site improvements; the presence of underground utilities; and the additional risk imposed by scope-of-service limitations imposed by the client. Your report should not be used if there are any changes to the project without first asking Coffey to assess how factors that changed subsequent to the date of the report affect the report's recommendations. Coffey cannot accept responsibility for problems that may occur due to changed factors if they are not consulted.

Subsurface conditions can change

Subsurface conditions are created by natural processes and the activity of man. For example, water levels can vary with time, fill may be placed on a site and pollutants may migrate with time. Because a report is based on conditions which existed at the time of subsurface exploration, decisions should not be based on a report whose adequacy may have been affected by time. Consult Coffey to be advised how time may have impacted on the project.

Interpretation of factual data

Site assessment identifies actual subsurface conditions only at those points where samples are taken and when they are taken. Data derived from literature and external data source review, sampling and subsequent laboratory testing are interpreted by geologists, engineers or scientists to provide an opinion about overall site conditions, their likely impact on the proposed development and recommended actions. Actual conditions may differ from those inferred to exist, because no professional, no matter how qualified, can reveal what is hidden by earth, rock and time. The actual interface between materials may be far more gradual or abrupt than assumed based on the facts obtained. Nothing can be done to change the actual site conditions which exist, but steps can be taken to reduce the impact of unexpected conditions. For this reason, owners should retain the services of Coffey through the development stage, to identify variances, conduct additional tests if required, and recommend solutions to problems encountered on site.

Your report will only give preliminary recommendations

Your report is based on the assumption that the site conditions as revealed through selective point sampling are indicative of actual conditions throughout an area. This assumption cannot be substantiated until project implementation has commenced and therefore your report recommendations can only be regarded as preliminary. Only Coffey, who prepared the report, is fully familiar with the background information needed to assess whether or not the report's recommendations are valid and whether or not changes should be considered as the project develops. If another party undertakes the implementation of the recommendations of this report there is a risk that the report will be misinterpreted and Coffey cannot be held responsible for such misinterpretation.

Your report is prepared for specific purposes and persons

To avoid misuse of the information contained in your report it is recommended that you confer with Coffey before passing your report on to another party who may not be familiar with the background and the purpose of the report. Your report should not be applied to any project other than that originally specified at the time the report was issued.

Interpretation by other design professionals

Costly problems can occur when other design professionals develop their plans based on misinterpretations of a report. To help avoid misinterpretations, retain Coffey to work with other project design professionals who are affected by the report. Have Coffey explain the report implications to design professionals affected by them and then review plans and specifications produced to see how they incorporate the report findings.



Important information about your **Coffey** Report

Data should not be separated from the report*

The report as a whole presents the findings of the site assessment and the report should not be copied in part or altered in any way. Logs, figures, drawings, etc. are customarily included in our reports and are developed by scientists, engineers or geologists based on their interpretation of field logs (assembled by field personnel) and laboratory evaluation of field samples. These logs etc. should not under any circumstances be redrawn for inclusion in other documents or separated from the report in any way.

Geoenvironmental concerns are not at issue

Your report is not likely to relate any findings, conclusions, or recommendations about the potential for hazardous materials existing at the site unless specifically required to do so by the client. Specialist equipment, techniques, and personnel are used to perform a geoenvironmental assessment. Contamination can create major health, safety and environmental risks. If you have no information about the potential for your site to be contaminated or create an environmental hazard, you are advised to contact Coffey for information relating to geoenvironmental issues.

Rely on Coffey for additional assistance

Coffey is familiar with a variety of techniques and approaches that can be used to help reduce risks for all parties to a project, from design to construction. It is common that not all approaches will be necessarily dealt with in your site assessment report due to concepts proposed at that time. As the project progresses through design towards construction, speak with Coffey to develop alternative approaches to problems that may be of genuine benefit both in time and cost.

Responsibility

Reporting relies on interpretation of factual information based on judgement and opinion and has a level of uncertainty attached to it, which is far less exact than the design disciplines. This has often resulted in claims being lodged against consultants, which are unfounded. To help prevent this problem, a number of clauses have been developed for use in contracts, reports and other documents. Responsibility clauses do not transfer appropriate liabilities from Coffey to other parties but are included to identify where Coffey's responsibilities begin and end. Their use is intended to help all parties involved to recognise their individual responsibilities. Read all documents from Coffey closely and do not hesitate to ask any questions you may have.

* For further information on this aspect reference should be made to "Guidelines for the Provision of Geotechnical information in Construction Contracts" published by the Institution of Engineers Australia, National headquarters, Canberra, 1987.

Appendix A - 68 Exploration Way Earthworks Specification

1. EARTHWORKS SPECIFICATION

1.1. General

The material on site is residually weathered to completely weathered greywacke (grade V and VI) and is generally suitable for fill. Earthworks are to be carried out as specified in NZS4431:1989, NZS4404:2010 and the PCC Code for Land Development and Subdivision Engineering.

The work under this section of the Specification includes all the earthworks necessary for general site clearing, excavations, stockpiling, foundation construction and site reinstatement.

Fill material has been tested from the stockpiles of excavated material from TP01 and TP03. The laboratory testing results are presented in Appendix D and have been used to develop this earthworks specification.

1.2. Soil Conditions

Where actual foundation/ subgrade conditions are found to vary from those indicated within this report, or on the specified drawings, or the foundation cannot be constructed as designed for any reason, Coffey should be notified before proceeding to allow an assessment to be made.

The nominated construction Contractor shall record the classification of the soil at the foundation / subgrade level in accordance with the following criteria, if Dynamic Cone Penetration (DCP) tests are not completed. This classification is with respect to the natural soil, not fill material.

The information shall be included on the as-built drawings:

1. Weak Soft Cohesive Soil – easily moulded by fingers. Equivalent DCP values < 5 blows / 300mm.
2. Medium Firm Cohesive Soil – moulded by strong pressure of fingers. Equivalent DCP values between 5 to 10 blows / 300mm.
3. Strong Stiff Cohesive Soil – Dented by strong pressure of fingers. Equivalent DCP values > 10 blows / 300mm.
4. Weak Loose Granular Soil – Easily penetrated with 12mm rod pushed by hand. Equivalent DCP < 10 blows / 300mm.
5. Medium Dense Granular Soil – Easily penetrated with 12mm rod driven with 2.3kg hammer. Equivalent DCP values 10 to 30 blows / 300mm.
6. Strong Dense Granular Soil – Penetrate 300mm with 12mm rod driven with 2.3kg hammer. Equivalent DCP values > 30 blows / 300mm.

1.3. Safety during Excavation

Excavations shall be protected at all times to eliminate any danger to persons or property, including livestock. The Contractor shall ensure that appropriate measures are used at all times to protect workers from the hazards of the excavation. The requirements of OSH “Approved Code of Practice for Safety in Excavations and Shafts for Foundations” shall be met as a minimum.

1.4. Excavations

All excavation work shall comply with the “Approved Code of Practice for Safety in Excavation and Shafts for Foundations”. An additional requirement is that all excavations at or below 1.5m in depth shall have appropriate trench support, if batters are not constructed. Coffey’s written approval is required to vary this requirement across the site.

Strutting, shoring, sheetpiles etc shall properly support excavations, or the side slopes should be cut back at an angle of 1V:2H. The excavations shall be kept clear of fallen material, slush and water. Provide adequate protection against surface erosion of all slopes around excavations and on the site in general. Ensure the safety of all existing installations including fences, structures and foundations.

Prior to any excavations, the Contractor shall clearly mark out the proposed excavation area. This mark shall be identifiable at all times, including after the excavation is finished. Therefore, the spray must be outside the excavation zone.

1.5. Stockpiling

Material stockpiles shall be located away from open cuts to avoid edge instability.

1.6. Fill Placement and Compaction Testing

The majority of earthworks will be cut to fill using reworked residually weathered to completely weathered Greywacke material from across the site. The Maximum Dry Density (MDD) of the residual soil/ completely weathered Greywacke is 1.66t/m³ and the Optimum Moisture Content (OMC) is 20%.

Fill material shall be placed at 95% MDD and with moisture content within approximately +/-2% of the OMC.

Wetting of the fill is therefore required to allow efficient compaction. Compacting the material below the OMC will not allow effective removal of air voids and conversely when attempting to compact material above the OMC propagation of ‘waves’ are likely to be observed from the roller as the material deforms without compaction.

The maximum thickness of layers placed for compaction (‘lifts’) and the number of passes by equipment is dependent on the type of compaction plant to be used and the weight of the compactor. For a smoothed wheeled roller (or vibratory roller operating without vibration) the lift thickness and number of passes required are provided in Table 2 below.

Table 1 Fill Compaction Requirements

Mass Per Metre Width of Roller*	Lift thickness (mm)	No. of passes
Over 2,100kg up to 2,700kg	125	10
Over 2,700kg up to 5,400kg	125	8

*Mass per metre width of roller is total mass of the roll divided by the total width of the roll.

Nuclear Densometer (NDM) testing should be undertaken to assess the soil density/ compaction of the completed placement and compaction of the first area of fill.

1.7. Inspection and Earthworks Supervision

To ensure that fill is placed to achieve desired results, and for Coffey to certify fill placed on site, it is envisaged that a minimum of 3 visits will be required for each lot or fill area to enable certification to be achieved. These visits are presented below:

1. Initial visit – this visit would occur following stripping of vegetation and topsoil, to inspect the subgrade material to ensure a sufficient bearing capacity exists. This includes completing DCPs and hand held Shear Vane testing to assess the subgrade material. If results indicate an inadequate bearing stratum, Coffey may recommend undercutting or the removal of soft spots. This initial inspection is to occur before placement of fill in the designated area being inspected.
2. Visit(s) during construction – it is envisaged that at least one visit will be required during fill placement to ensure that the methodology of placement is sound and adequate to the material being placed. It is expected that a single visit covers up to one vertical metre of fill being placed. In locations of thicker fill, more visits may be required. During the visit, it is expected that observations of Nuclear Densometer Testing (NDTs) will be undertaken to ensure that these are as per the earthworks specifications provided. Coffey understands that Progeni will arrange for the appropriate Contractors to complete these in situ tests.
3. Final visit – a final visit will be undertaken when Progeni informs Coffey that the filling works for a specific area are coming to a close, to allow certification.

To reduce costs incurred due to travel, it is recommended that multiple fill sites are prepared to allow a number of inspections (across varying lots/ fill locations) to be ready for certification from both Coffey and the soil testing specialist (if/ when required).

1.8. Subgrade Assessments

At locations where fill is to be placed and at building locations an inspection should be undertaken when stripping of the vegetation and topsoil is completed. This can be completed for multiple sites at once, to allow a number of lot inspections to be completed in one trip.

Once each site is stripped, DCPs and shear vanes are to be completed to assess the subgrade before fill placement, and to identify any soft spots. Generally, once stripping is complete, fill works are completed immediately to avoid the potential of regrowth of vegetation or drying out of the subgrade.

1.9. Existing Fill

There are some areas of existing fill on the site in Lots 5, 6 and adjacent to Lot 10. At Lot 5, the fill is very loose to loose silt that is currently at a 28° to 42° slope and appears to be creeping and slumping. This will need to be excavated and re-compacted/ replaced prior to lot development (site construction) as it is currently unstable.

1.10. Topsoil and Grassing

Excavated topsoil from the site can be re-used for topsoiling. Topsoil should be stockpiled in a free draining area of the site and away from trafficable areas to avoid compacting the material.

1.11. Permanent Slopes

The construction of permanent slopes across the site of less than 6 meters high are to be at a maximum angle of 1 vertical to 1.5 horizontal (33/34°) as specified in the PCC Code of Land Development and Subdivision Engineering. The upper 1 meter of cuts is to be rounded off.

Steeper slope cuts are proposed for the northern part portion of Lot 7 adjacent the right of way (ROW) and adjacent the ROW at Lot 8. Specific Engineering Design (SED) is required to assess the stability of these slope cuts and maximum appropriate slope angle. A numerical slope stability assessment is recommended at the most critical slope cut cross section (i.e. steepest, highest) which can then be used to assess stability of the remainder of the steepened cuts. The soil conditions found during the

excavation of the test pit can be used to inform the slope stability assessment which can be undertaken as a variation to the existing contract between Coffey and Progeni Limited.

Areas of fills to be at a maximum angle 1 vertical to 2 horizontal (26°) as specified in the PCC Code of Land Development and Subdivision Engineering C2.12; or alternatively, SED will be required if fill slopes are steeper than 26°.

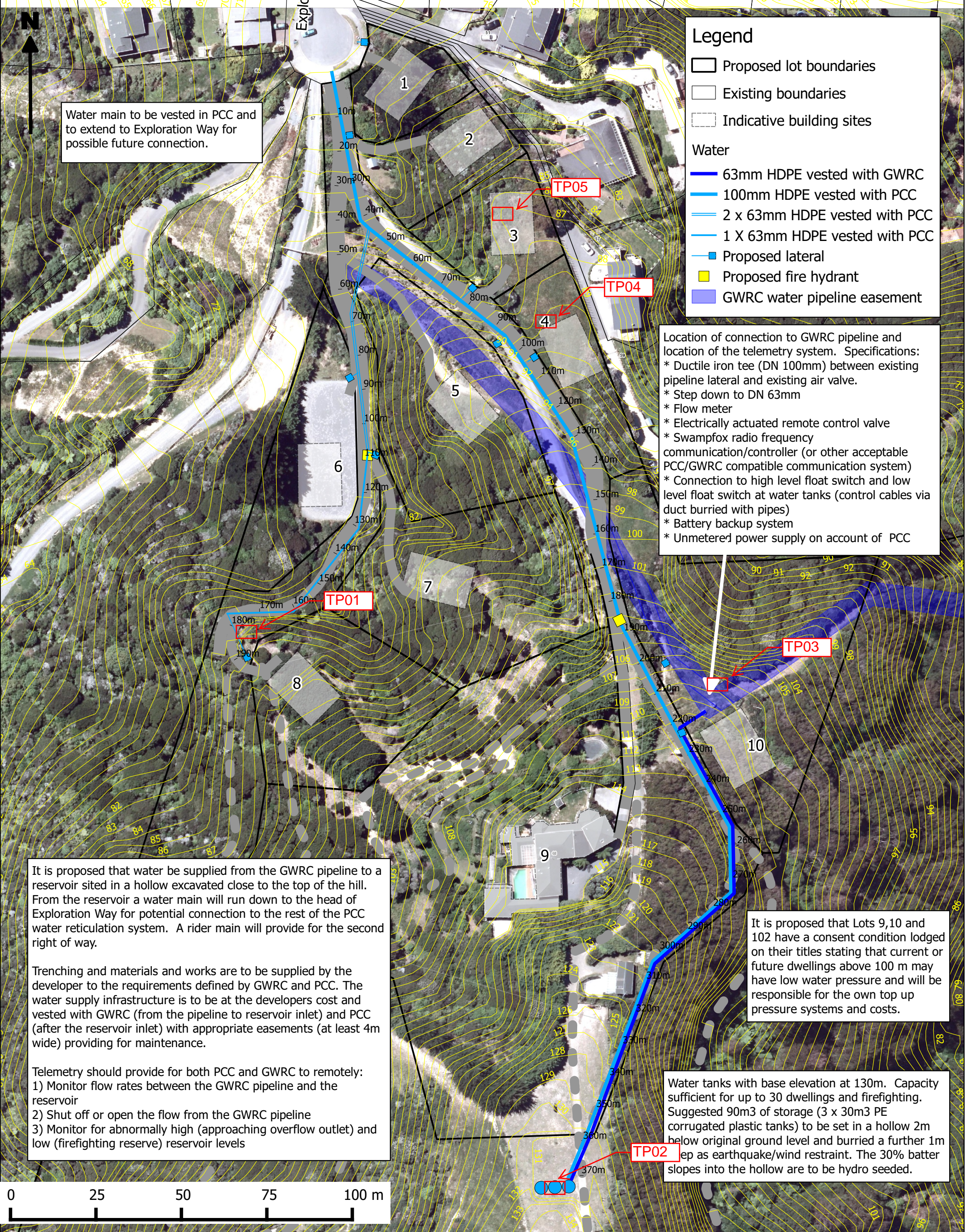
2. REFERENCES

Devon, Installation Guide - Devan Water Tanks

http://www.devon.co.nz/user_files/Website%20Downloads/Tank%20installation%20-%20Email.pdf

Porirua City Council Code of Land Development and Subdivision Engineering (February 2010)

Attachment A - Test Pit Location Plan




<div>Progeni Ltd</div> <div>68 Exploration Way, Porirua 5024 Ph 64 022 0819600</div> <div>Copyright 2013 Progeni Ltd. All rights reserved</div>	Project: Subdivision of Lot 2 DP 358205 and Lot 1 DP 426821	Approvals:				Versions:			
	Title: Water reticulation layout	Date:	Function:	By:	Signature:	Rev:	Amendment:	By:	Date:
		2013-10-28	Design	DWH		A	Initial issue Move west ROW	DWH	2013-10-28
		2013-10-28	Drawn	DWH		B		2013-12-14	
	Checked								
	Checked			Drawing No: 2013-P1-H Rev B A3 Scale 1:1000					

Attachment B - Test Pit Photographs




TP01

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approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP01		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 1
original size	A4				rev:	




TP01

drawn	AH		client:	Progeni Limited		
approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP01		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 2
original size	A4				rev:	




TP02

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approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP02		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 3
original size	A4				rev:	




TP02

drawn	AH		client:	Progeni Limited		
approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP02		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 4
original size	A4				rev:	




TP03

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approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP03		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 5
original size	A4				rev:	




TP03

drawn	AH		client:	Progeni Limited		
approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP03		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 6
original size	A4				rev:	




TP04

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approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP04		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 7
original size	A4				rev:	




TP04

drawn	AH		client:	Progeni Limited		
approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP04		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 8
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TP05

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approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP05		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 9
original size	A4				rev:	



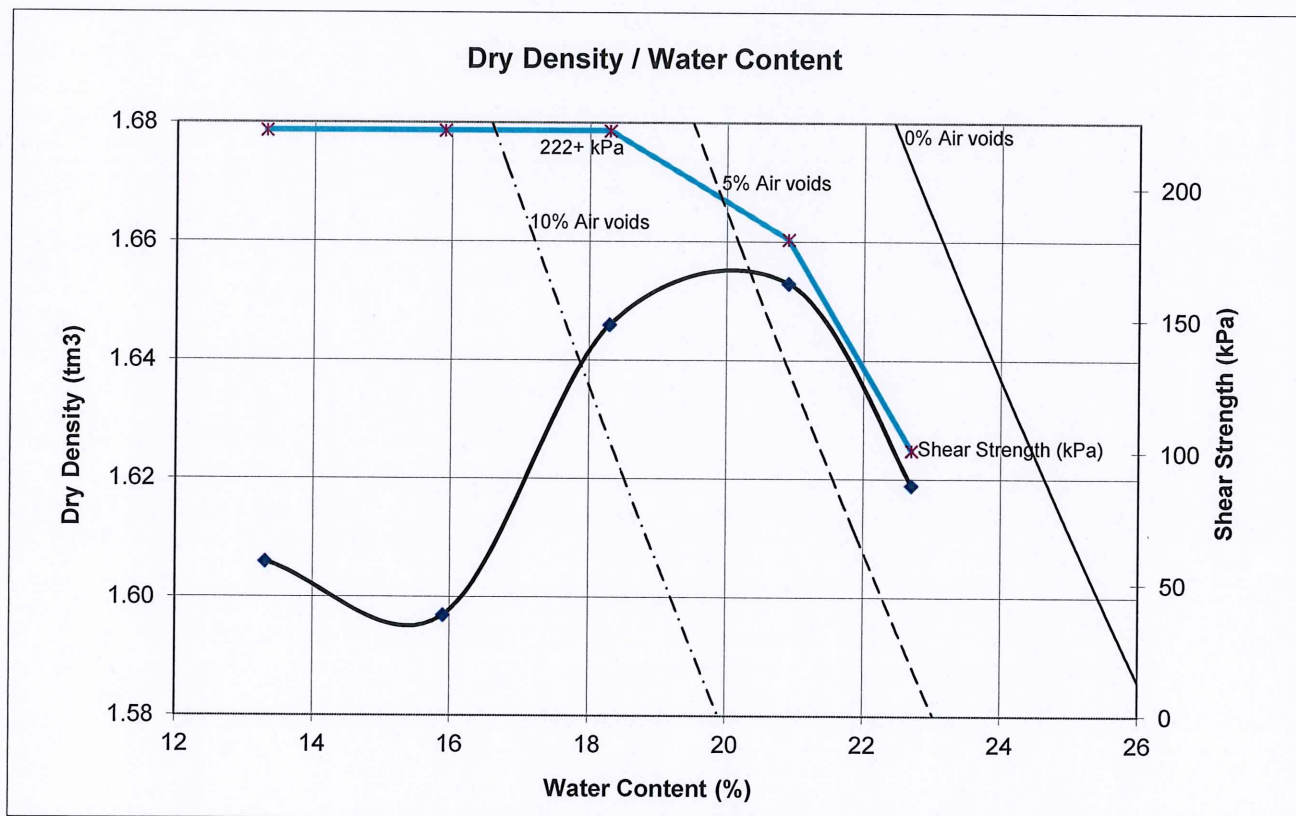
TP05

drawn	AH		client:	Progeni Limited		
approved	KWH		project:	68 Exploration Way Geotechnical Investigation		
date	02/04/2015		title:	TEST PIT PHOTOGRAPH TP05		
scale	N.T.S.		project no:	GENZWELL16067AA	fig no:	FIGURE 10
original size	A4				rev:	

Attachment C - Laboratory Test Results

**DETERMINATION OF THE DRY DENSITY / WATER CONTENT RELATIONSHIP
NEW ZEALAND STANDARD COMPACTION
TEST METHOD NZS 4402 : 1986 TEST 4.1.1**

Job:	GENZWELL16067AB - 68 EXPLORATION WAY WHITBY - FURTHER WORK		
Date of order:	13/02/2015	Sample No.:	ETAM15S-01323
Sample method:	Unknown (Not IANZ Endorsed)	Sample Origin :	Test Pit 1 & 3
Sampled By :	Client	Sample Date :	Delivered 13/02/2015
Client:	Coffey Geotechnics NZ Ltd, Wellington	Attention:	Andrew Hutchinson



Maximum Dry Density :	1.66	t/m³	Optimum Water Content :	20	%
Natural Water Content :	9.9	%			
Solid Density of Soil :	2.7	t/m³			
Description of Soil :	Clayey SILT, yellow-brown (Not IANZ Endorsed)				
Fraction of soil tested :	Passing 19mm sieve		History of sample :	Natural	
Comments :	Shear Vane test method - NZ Geotechnical Society Inc 2001				

Tested By:	KB	Date :	17/02/2015
Calculated By :	JM	Date :	18/02/2015
Checked By :	JM	Date :	19.2.15

Approved Signatory :		Date :	19.2.15
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Appendix B - Water Reservoir Platform Slope Stability Assessment

1. WATER RESERVOIR PLATFORM ASSESSMENT

1.1. General

Initially three 30,000L water reservoirs (tanks) are proposed to be installed at the site, with the prospect of up to eight tanks being installed in the future.

The water tanks will be housed in a cut measuring approximately 20m by 20m at the top of the slope on the southern portion of the site. The tanks will be set in a hollow two metres below current ground level. The tanks manufacturer installation guide (Devan, undated) specifies that the tanks be founded on a 100mm thick sand base with 250mm clearance from the edge of the tank to the natural soil.

The tanks are typically 3.7m diameter and weigh 30.475 tonnes when full (includes tank weight of 0.475t). A test pit (TP02) has been excavated in the area approximately central to the platform to be cut out of the slope crest. The test pit found the ground to comprise a 0.5m thick layer of topsoil underlain by orange/brown completely weathered greywacke.

1.2. Subgrade Assessment

The subgrade for the water tank, being residual Greywacke soil is likely to have an ultimate bearing capacity of over 1,000kPa which exceeds the pressure exerted by a 30,000L water tank, calculated to be in the order of 42kPa (factored load). Therefore the subgrade of the tank farm site is considered to be adequate for the imposed loads of up to eight water tanks.

1.3. Slope Stability Assessment

The stability of the more critical steeper slope to the east of the proposed water storage tank reservoir has been assessed taking into account the loads from the three water tanks which have been assumed to be full (load 42kPa per tank). Stability has been calculated for the following conditions:

1. Prevailing groundwater level at 5.4m below ground level.
2. Transient elevated ground water level at 3.8m below ground level.

The results of the assessment are given in terms of Factor of Safety (FoS) against failure. Material parameters have been back calculated assuming the slope is stable at its present angle of 46° i.e. FoS >1. Modelling shows the most critical slip circle is a shallow failure in residual Greywacke soil through the slope surface which has a FoS of 1.09.

Slip circles intersecting the underside of the tanks have then been inspected to allow an assessment of the influence of the tank farm on the stability of the slope. A FoS of >1.5 is assessed as stable for the prevailing groundwater condition and a factor of safety of >1.3 is assessed as stable for the transient elevated groundwater condition.

Table 2 below provides both the back calculated FoS for the slope and the slope stability assessment with the imposed loads from the water tanks. Output models are provided in Attachment A. The slope stability analysis indicates that the slope is still stable under the imposed loads from the water tanks.

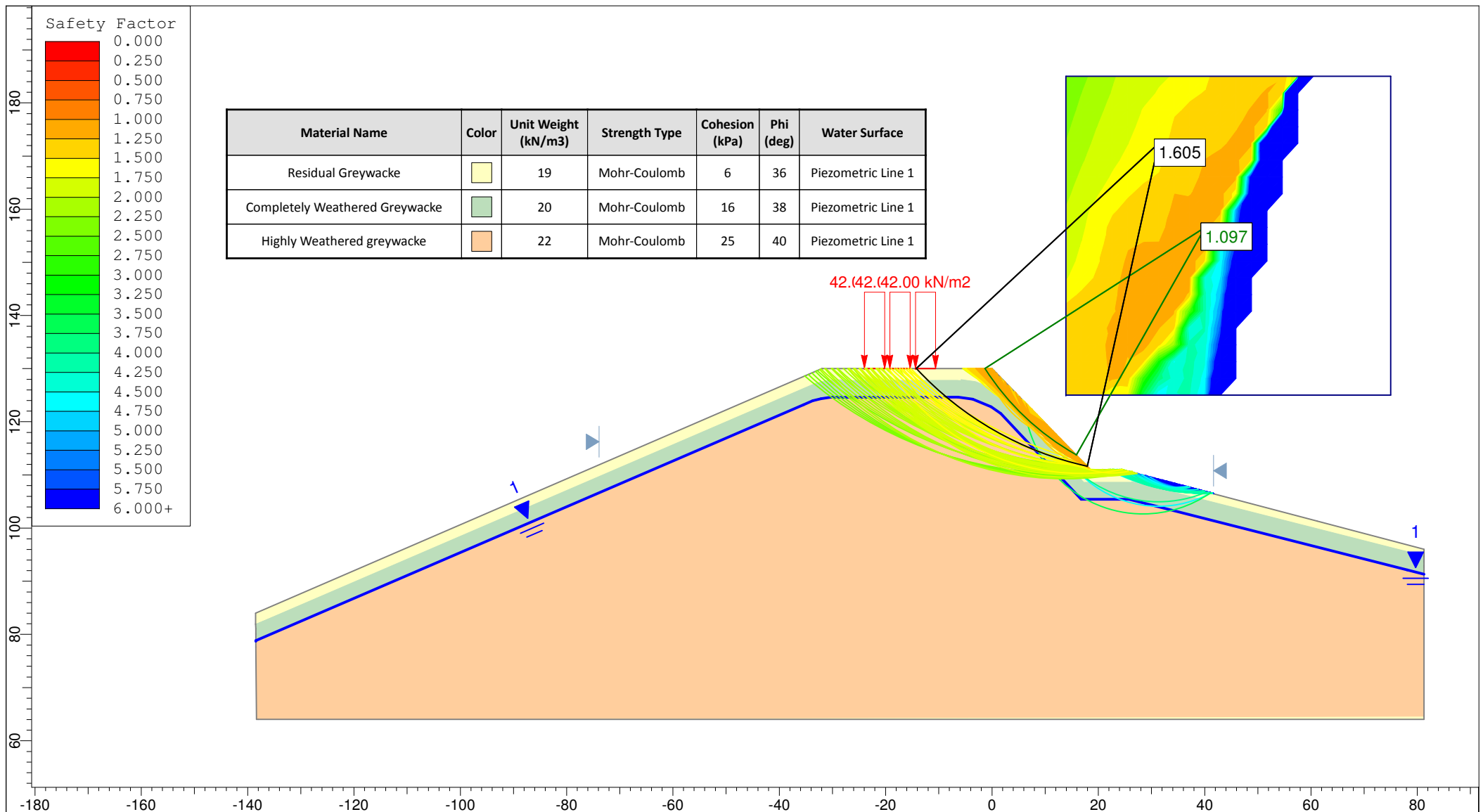
Table 1 Water Reservoir Slope Stability Assessment Summary

Scenario	Required FoS	Minimum FoS Beneath Water Reservoir
Prevailing Ground Water Level	1.50	1.61
Transient Elevated Ground Water Level	1.30	1.36

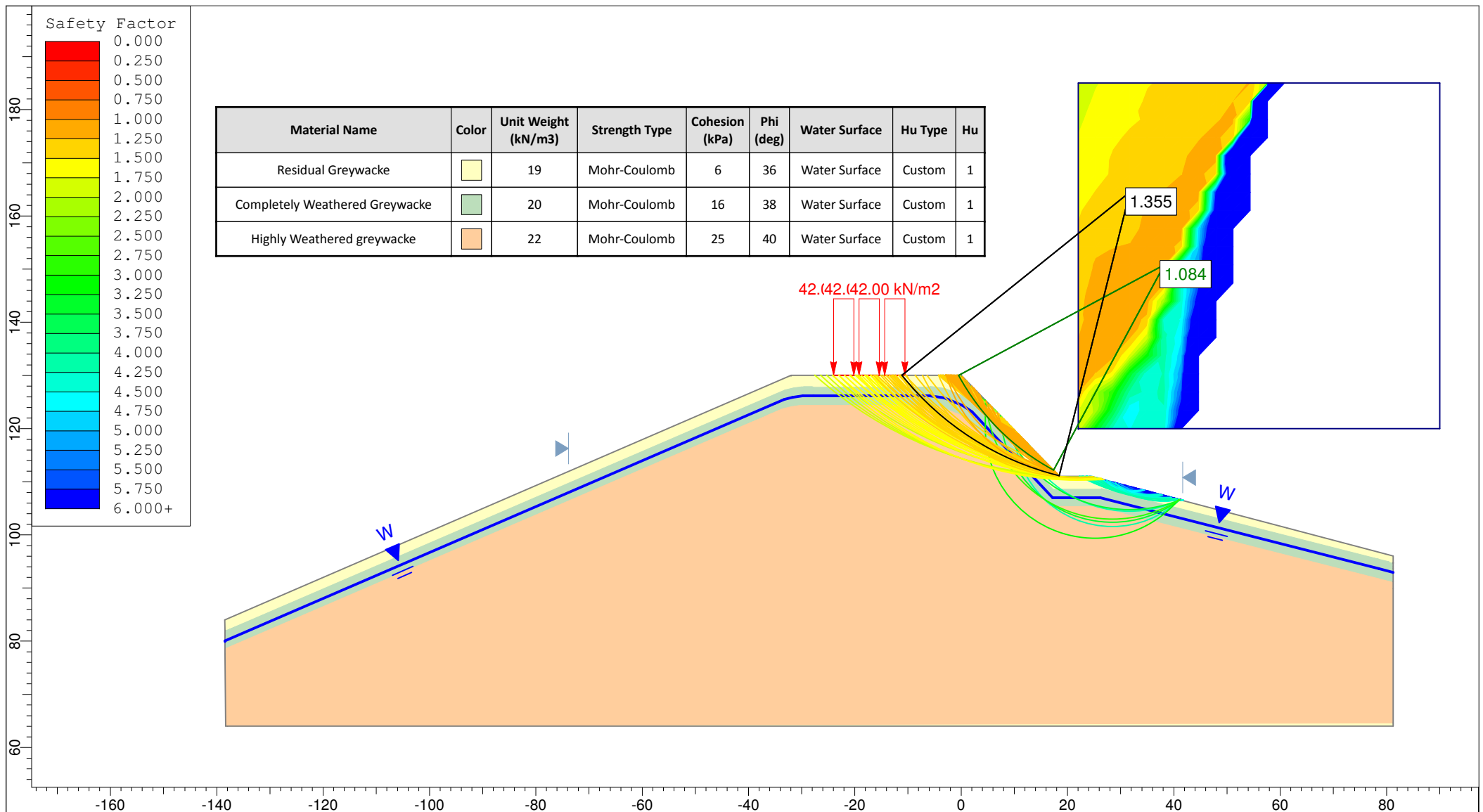
Attachment A – Slope Stability Modelling Outputs

Prevailing Groundwater Level

Transient Elevated Groundwater Level



Client		Coffey			
Project		68 Exploration Way Water Reservoir Stability Analysis			
Analysis Description		Prevailing Ground Water Level			
Drawn By	AH	Approved By	AH	Scale	1:1000
Date	1/04/2015, 10:41:49 a.m.	Figure	NS	Project no.	GENZ
				File Name	Water Reservoir Statbility - PGW_rev2.slim



Client		Coffey		
Project		68 Exploration Way Water Reservoir Stability Analysis		
Analysis Description		Transient Elevated Ground Water		
Drawn By	AH	Approved By	AH	Scale
Date	1/04/2015, 10:41:49 a.m.	Figure	NS	Project no.
				GENZ
				File Name
				Water Reservoir Statbility - TEGW_rev2.slim